Southwest Basins No. 2 & No. 3 Sanitary Sewer System Study

Prepared for the City of Franklin Engineering Department August 2014



Prepared by:





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I. Executive Summary

This study was authorized on March 25, 2014 by the Franklin Engineering Department in order to evaluate and update the expected sewer demand in the Southwest Basins No. 2 and No. 3. This evaluation uses data from the 2011 Development Report that was compiled by the Department of Planning and Sustainability, provided by the Engineering Department, and as-built drawings from the Waste Management Department. This information was implemented to produce expected sewer flows from Southwest Basins No. 2 and No. 3. The findings, conclusions, and recommendations for the two basins are detailed in the following report.

II. Description

i. Southwest Basin No. 2

The Southwest Basin No. 2 is located in the southwestern sections of the City of Franklin's Urban Growth Boundary (UGB). The borders are roughly comprised of: Coleman Road to the north, Goose Creek Bypass to the east, Snowbird Hollow Road and West Harpeth River to the south, and Kelly Branch to the west. The exact location and boundaries are shown in Exhibit B-1.

The Southwest Basin No. 2 is comprised of approximately 2,045 acres, in which 1,441 acres are undeveloped. There are currently 197 existing lots with capacity to expand to 2,359 projected lots. The fully developed basin is anticipated to have a population of 6,486 residents.



Based on the natural terrain of the Southwest Basin No. 2, it has been divided into two distinct sub-basins. The area does not currently offer gravity sewer service to its residents.

ii. Southwest Basin No. 3

The Southwest Basin No. 3 is located in the southwestern sections of the City of Franklin's UGB. The borders are roughly comprised of: Snowbird Hollow Road to the north, Lewisburg Pike to the east, West Harpeth River to the south, and Hunter Road to the west. The exact location and boundaries are shown in Exhibit B-1.

The Southwest Basin No. 3 is comprised of approximately 2,092 acres, in which 1,135 acres are undeveloped. There are currently 231 existing lots with capacity to expand to 1,933 projected lots. The fully developed basin is anticipated to have a population of 5,316 residents.

Based on the natural terrain of the Southwest Basin No. 3, it has been divided into three distinct sub-basins. The area does not currently offer gravity sewer service to its residents.

III. <u>Development of Sanitary Sewer Flows</u>

i. Rationale

The two Basins flow to a common point and were analyzed together to size the gravity sewer lines. Figures 1 and 2 lists the various areas delineated within the sub-basins of Southwest Basin No. 2 and Southwest Basin No. 3 respectively. Existing information from the 2011 Development



Report and as-built drawings were utilized to determine land use within each basin.

Each sub-basin was evaluated in terms of land use, and Franklin Planning Department estimates and projections were used to identify the type and extent of future development for properties where no submittals have been submitted.



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	FIGURE 1 - SOUTHWEST BASIN NO. 2 LAND USE	F BASIN N	O. 2 LAND	USE	
Area	Description	Total	Total Undeveloped	No. Of Estimated	
Number		Acreage	Acreage	Developed Lots	In Area
1	Kittrell Rd and Hunter Rd Intersection	1,545	1,195	1,792	1,843
2	Hunter Rd to Columbia Pk	200	246	698	515
	Totals	2,045	1,441	2,162	2,359

	FIGURE 2 - SOUTHWEST BASIN NO. 3 LAND USE	BASIN N	0. 3 LAND	USE		
Area	Description	Total	Total Undeveloped	No. Of Estimated	Est. Total Developed Lots	
Number		Acreage	Acreage	Developed Lots	In Area	_
7	Hunter Rd, Columbia Pk, and Snowbird Hollow Rd	868	452	678	767	_
						_
2	Goose Creek Bypass	629	199	298	389	_
						_
3	Tom Anderson Rd	615	484	726	777	_
	Totals	260.2	1,135	1,702	1.933	-



The State of Tennessee Sewage Works Design Manual provides guidance for incremental flow for various land uses. The Manual recommends a flow of 350 gallons/day (gpd) for a single family residence. Every property in the two Basins were understood to be zoned for single family residence and the flow projections are based on the recommended flow.

For the purpose of this study, flood plains, steep slopes and roads were included in the undeveloped acreage calculation. Average projected flows from each sub-basin are calculated from the general information presented in Figures 1 and 2 above. Sanitary sewer lines must be designed for the maximum flow that will be encountered in a given area within the drainage basin. Peaking Factors are applied that allow for the differences in average daily flows and instantaneous sewer flows. These factors vary according to the number of people in the sub-basin contributing to the flow. The following peaking factors listed in Figure 3 are taken from ASCE "Sewer Design & construction Manuals and Reports on Engineering Practices" and are used in this study.

FIGURE 3 - POPULATI	ON PEAKING FACTORS
Tributary Population	Peaking Factor
0-500	5
501-1000	4
1001+	3.25

ii. Projected Flows

Figure 4 and 5 represent an overview of the entire land development plan for the Southwest Basin No. 2 and No. 3, respectively. A more detailed analysis of the information contained in Figure 1, 2, 4, and 5 is contained



in Table 1 and Table 2 in the Appendix. Table 1 contains projected flows associated with each sub-basin for Southwest Basin No. 2 and Table 2 contains the project flows for Southwest Basin No. 3. The information in the tables is based on the information from the 2011 Development Report and counts of actual properties on the as-built drawings.

The calculations included in the tables project numbers of future residential units and sewage flow from each sub-basin attributable to the proposed development. A majority of the information was derived from population densities. The City of Franklin's land use plan calls for a residential density of 1.0 unit per acre. After further review a value of 1.5 residential units per acre was used based on the existing high density subdivisions in the basins and at the request of the City of Franklin.



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		No. Of	Est. Total	Est. Population	Est. Total	Total Average	Total Peak
Description	Total	Estimated	Developed Lots		Population	Flow	Flow
	Acreage	Acreage Developed Lots	In Area	2.75/unit	In Area	(MGD)	(MGD)
Kittrell Rd and Hunter Rd Intersection	1,545	1,792	1,843	4,928	5,069	0.645110	2.096606
ia Pk	200	369	515	1,016	1,418	0.180413	0.586341
Totals	2,045	2,162	2,359	5,944	6,486	0.825522	2.682947
-						-	
	ittrell Rd and Hunter Rd Intersection unter Rd to Columbia Pk Totals		500 2,045	500 369 2,162 2,045 2,162	on 1,545 1,792 1,843 500 369 515 2,045 2,162 2,359	on 1,545 1,792 1,843 4,928 500 369 515 1,016 2,045 2,162 2,359 5,944	on 1,545 1,792 1,843 4,928 5,069 500 369 515 1,016 1,418 2,045 2,162 2,359 5,944 6,486

	FIGURE 5 - SOUTH	IWEST	5 - SOUTHWEST BASIN NO. 3 LAND USE and FLOW RESULTS	3 LAND USE	E and FLOW	/ Results	•	
			No. Of	Est. Total	Est. Population		Est. Total	Total Peak
	Description	Total	Estimated	Developed Lots	In Area	Population	Flow	Flow
Number		Acreage	Acreage Developed Lots	In Area	2.75/unit	In Area	(MGD)	(MGD)
1 7	Hunter Rd, Columbia Pk, and Snowbird Hollow Rd 898	868	829	192	1,865	2,110	0.268555	0.872804
	2 Goose Creek Bypass	629	867	389	819	1,069	0.136115	0.442374
. –1	3 Tom Anderson Rd	615	726	777	1,996	2,136	0.271898	0.883667
	Totals	2 092	1 702	1 933	4 681	5.316	895929 0	2 198844



The average daily flow for the Southwest Basin No. 2 is estimated to have an average of 0.826 million gallons per day (MGD) and the Southwest Basin No. 3 estimated average is 0.677 MGD. The average flows will have a significant increase to 2.683 MGD and 2.199 MGD, respectively, after applying peaking factors and the two flows combine to equal 4.882 MGD.

iii. Existing Interceptor Sewers

Previous sections have provided a rationale for projections of sewer flows from the study basins. With the flows from each sub-basin defined, existing sanitary sewer lines can be evaluated, areas of insufficient capacity can be identified, and specific infrastructure improvements can be recommended. To develop Southwest Basins No. 2 and No. 3 the flow will have to be conveyed through the Goose Creek sanitary sewer that reaches out to Berry Farms and flows into the Five Mile Creek Interceptor. This is the most logical tie-in location in relation to distance and existing infrastructure capable of handling the flow from the two basins. Figure 6 below shows the size and slope of the existing sewer, and the full flow capacities. City of Franklin provided the as-built drawings from Littlejohn Engineering Associates to analyze the existing infrastructure. The existing sewer material is PVC and a Manning's roughness value of n = 0.011 was used to calculate capacity.



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FIGURE 6 - E	XISTING GOOSI	E CREEK GRA	VITY SEWER
SHEET	PVC SEWER DIAMETER	SLOPE	FULL FLOW CAPACITY, MGD
	18-inch	0.59%	6.176
C2.2	18-inch	0.68%	6.631
C2.2	18-inch	0.55%	5.979
	18-inch	0.75%	6.955
	18-inch	0.46%	5.441
00.4	18-inch	0.58%	6.111
C2.1	18-inch	0.85%	7.404
	18-inch	0.66%	6.524
	18-inch	1.49%	9.789
	18-inch	0.62%	6.309
C2.0	18-inch	0.45%	5.375
C2.0	18-inch	0.75%	6.964
	18-inch	1.09%	8.380
	18-inch	0.79%	7.138

A study of the Goose Creek sub-basin, that the flow will be conveyed through, was conducted to develop the estimated peak flow from the area which is projected at 3.17 MGD. This projection came from the existing 322 units, 2.8 million square feet of commercial zoned areas, and the possible development of 254 acres. The combined projected flows for the Goose Creek sub-basin and Southwest Basin No. 2 and No.3 have a peak flow of 8.05 MGD. After analyzing the full flow capacities of the existing sewer, it is anticipated that the existing gravity sewer will not be able to handle the total combined flow and a solution is described in the following paragraphs.

The terrain between the Berry Farms gravity sewer and the common low area for the Southwest Basins, has elevation changes that causes a problem. This problem of having the water carried between the two points left only one option and that was to pump the water. The selected site for the proposed pump station is located in the southwest corner of Southwest Basin No. 2 and sits close to the West Harpeth River. This



location was selected based on the contour lines and the ability to have the Southwest Basins on gravity sewer. All proposed gravity sewers are anticipated to be able to serve all areas of the basins. A 16-inch force main will need to be constructed to convey the flow from the proposed pump station to Berry Farms sanitary sewer.

IV. Option Descriptions

There are three options to consider when conveying flows from Southwest Basin No. 2 and No. 3 to the Goose Creek sub-basin.

i. Option1: Force Main to GCB with New 27" Line and 18" Extension

Option 1, shown in Exhibit B-2, is to construct the force main to the intersection of Snowbird Hollow Road and Goose Creek Bypass, then lay a 27-inch line along the south side of the Goose Creek Bypass to the Five Mile Creek Interceptor. This will be used to convey the flows from the two Southwest Basins and lots on the southern boundary of the Goose Creek Bypass. An extension of the existing 18-inch Berry Farms sewer will be required to serve the northern boundary of the Goose Creek Bypass. This extension was calculated using the existing slope of the Berry Farms sewer. The total length of 16-inch force main is 17,398 linear feet, the proposed 27-inch line length is 10,749 linear feet and the extension of gravity sewer is 2,766 linear feet. The projected total flow to be handled is 8.05 MGD.



ii. Option 2: Force Main to GCB with New 24" Line and 24" Expansion

Option 2, shown in Exhibit B-3, is slightly different than Option 1 were a 24-inch line will run along the northern boundary of the Goose Creek Bypass until it reaches the existing Berry Farms 18-inch gravity sewer. A minimum slope was assumed for the 24-inch proposed gravity sewer. Do to the projected flows, the existing Berry Farms 18-inch sewer will need to be upsized to a 24-inch gravity sewer. Existing slopes were utilized to size the proposed 24-inch gravity sewer. This option will enable the properties north of the Goose Creek Bypass to be on gravity sewer while the southern properties will remain on septic systems. The projected quantities are: 17,398 linear feet for the 16-inch force main and 10,568 linear feet of new 24-inch gravity sewer, where 3,661 linear feet of 24-inch gravity sewer will replace the existing 18-inch Berry Farms sewer. The projected total flow to be handled is 5.56 MGD.

iii. Option 3: Force Main to Berry Farms Gravity Sewer

Option 3, shown in Exhibit B-4, is to construct the 16-inch force main the entire length to the existing 18-inch Berry Farms sewer. This will give the City of Franklin gravity sewer in the two Southwest Basins and the existing and projected area of Berry Farms Town Center. The projected density of 3.75 units per acre, for the Berry Farms Town Center, was obtained through the 2011 Development Report. The Berry Farms Town Center area to be developed is estimated at 86 acres. The existing and projected units are estimated to have a peak flow of 0.89 MGD, combined with the projected 4.88 MGD from the Southwest Basins, equals a total peak flow of 5.77 MGD. The current capacity of the Berry Farms gravity sewer has



two sections that are under sized for this amount but the overall capacity is estimated to handle the additional flow.

V. Recommendations

All three options have pros and cons when deciding which option is more suitable for the City of Franklin's needs. The price difference between the three options is estimated at \$2.4 million dollars and a breakdown of the cost estimates can be found below in Figure 7, 8, and 9. Option 1, on Exhibit B-2, will give the City of Franklin the ability serve the Southwest Basins and the entire Goose Creek sub-basin but is higher in cost. Option 2 will also allow the City to serve the Southwest Basins but will only have the capability of serving the northern portion of the Goose Creek sub-basin. The estimated 43 properties along the southern boundary of the Goose Creek Bypass will remain on septic systems. Option 3 requires a force main to be constructed across a span of almost five miles but is estimated to cost less and will not serve the portions of the Goose Creek sub-basin.

All three options share a common flaw that will require periodic flushing of the 16-inch force main to stir up the sediment in the line to produce more favorable conditions for an unknown period of time while development takes place. As shown in Table 1 and 2, in the Appendix, the existing, combined peak flow for Southwest Basin No. 2 and No. 3 is 749,000 gallons. This flow will have an estimated velocity of 0.830 feet per second (fps) which is not enough to keep the sediment suspended and the flushing will have to be continued until a flow of 1.8 MGD can be maintained. An additional option the City may want to consider is constructing two (2) parallel force mains to handle the flows from the Southwest Basins. Due to higher construction costs this option was not investigated further.

After reviewing the options for Southwest Basins No. 2 and No. 3, it will be more beneficial for the City to lay the proposed 27-inch line and extend the existing 18-inch gravity sewer from Berry Farms to allow for present and future residents to be served around the Goose Creek Bypass. The cost difference for this option is minimal while maximizing service in the Goose Creek sub-basin. Should this area be proposed to develop with greater densities, the City will need to reevaluate the basins in this area and update as necessary.

FIGURE 7 - ENGINEERS OPINION OF PROBABLE COST						
OPTION 1 - FORCE	MAIN TO	GC	В, С	GRAVITY '	ΤΟ	BF & FMC
Description	Quantity /	Unit		Unit Cost		Total
8-inch Gravity Sewer	35,166	LF	\$	86.00	\$	3,024,276.00
10-inch Gravity Sewer	14,270	LF	\$	95.00	69	1,355,650.00
12-inch Gravity Sewer	4,235	LF	\$	110.00	\$	465,850.00
15-inch Gravity Sewer	14,000	LF	\$	150.00	69	2,100,000.00
18-inch Gravity Sewer	7,615	LF	\$	200.00	\$	1,523,000.00
24-inch Gravity Sewer	1,127	LF	\$	225.00	\$	253,575.00
27-inch Gravity Sewer	10,749	LF	\$	240.00	\$	2,579,760.00
Manholes	210	EΑ	\$	4,250.00	69	894,285.00
4.9 MGD Pump Station	1	LS	\$2	,750,000.00	69	2,750,000.00
16-inch Force Main	17398	LF	\$	175.00	\$	3,044,650.00
SUBTOTAL					\$	17,991,046.00
10% Misc. Allow. For Bypass, Mobilization, etc.	1	LS	\$1	,799,104.60	\$	1,799,104.60
15% Contingency Allow. For Legal, Engineering, Admin. & Esmts.	1	LS	\$2	,698,656.90	\$	2,698,656.90
TOTAL					\$	22,489,000.00



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FIGURE 8 - ENGINEERS OPINION OF PROBABLE COST						
OPTION 2 - FORCE	MAIN TO	GC	В, С	SRAVITY '	ΤΟ	BF & FMC
Description	Quantity /	Unit		Unit Cost		Total
8-inch Gravity Sewer	35,166	LF	\$	86.00	\$	3,024,276.00
10-inch Gravity Sewer	14,270	LF	\$	95.00	\$	1,355,650.00
12-inch Gravity Sewer	4,235	LF	\$	110.00	\$	465,850.00
15-inch Gravity Sewer	14,000	LF	\$	150.00	\$	2,100,000.00
18-inch Gravity Sewer	4,849	LF	\$	200.00	\$	969,800.00
24-inch Gravity Sewer	11,695	LF	\$	225.00	\$	2,631,375.00
Remove Existing 18-inch	3,661	LF	\$	41.00	\$	150,101.00
Manholes	210	EΑ	\$	4,250.00	\$	894,285.00
4.9 MGD Pump Station	1	LS	\$2	,750,000.00	\$	2,750,000.00
16-inch Force Main	17,398	LF	\$	175.00	\$	3,044,650.00
SUBTOTAL					\$	17,385,987.00
10% Misc. Allow. For Bypass, Mobilization, etc.	1	LS	\$1	,738,598.70	\$	1,738,598.70
15% Contingency Allow. For Legal, Engineering, Admin. & Esmts.	1	LS	\$2	,607,898.05	\$	2,607,898.05
TOTAL					\$	21,733,000.00

FIGURE 9 - ENGINEERS OPINION OF PROBABLE COST						
OPTION 3 - FORCE	MAIN TO BE	RRY FARMS	GRAVITY SEWER			
Description	Quantity / Unit	Unit Cost	Total			
8-inch Gravity Sewer	35,166 LF	\$ 86.00	\$ 3,024,276.00			
10-inch Gravity Sewer	14,270 LF	\$ 95.00	\$ 1,355,650.00			
12-inch Gravity Sewer	4,235 LF	\$ 110.00	\$ 465,850.00			
15-inch Gravity Sewer	14,000 LF	\$ 150.00	\$ 2,100,000.00			
18-inch Gravity Sewer	4,849 LF	\$ 200.00	\$ 969,800.00			
24-inch Gravity Sewer	1,127 LF	\$ 225.00	\$ 253,575.00			
Manholes	210 EA	\$ 4,250.00	\$ 894,285.00			
4.9 MGD Pump Station	1 LS	\$2,750,000.00	\$ 2,750,000.00			
16-inch Force Main	24572 LF	\$ 175.00	\$ 4,300,100.00			
SUBTOTAL			\$ 16,113,536.00			
10% Misc. Allow. For Bypass, Mobilization, etc.	1 LS	\$ 1,611,353.60	\$ 1,611,353.60			
15% Contingency Allow. For Legal, Engineering, Admin. & Esmts.	1 LS	\$ 2,417,030.40	\$ 2,417,030.40			
TOTAL			\$ 20,142,000.00			



APPENDIX

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TABLE A-1

		SOUTHV	VEST BA	SIN NO. 2	SOUTHWEST BASIN NO. 2 SEWER CALCULATIONS	ALCULAT	SNOL				
		DEVE	LOPMEN	T OF SAN	DEVELOPMENT OF SANITARY SEWER FLOWS	VER FLO	WS				
			Total	No. Of	No. Of Lots		Population			Total Average	Total Peak
Area	Description	Total	Undeveloped	Existing	Planned	Prop. Density	In Area	Ave. Flow Peaking	Peaking	Flow	Flow
Number		Acreage	Acreage	Developed Lots	Developed Lots (Planning Dept.)	(Units/ Acre)	2.75/unit	(GPD)	Factor	(MGD)	(MGD)
-	Kittrell Rd and Hunter Rd Intersection	1,545	1,189	51		2	140	17,850	2	0.017850	0.089250
	1.A	229	205	3	0	1.5	8	1,050	2	0.001050	0.005250
	1.8	727	541	38	0	1.5	105	13,300	2	0.013300	0.066500
	1.0	440	294	10	0	1.5	28	3,500	2	0.003500	0.017500
	1.D	101	101	0	0	0	0	0	0	0	0
	1.E	48	48	0	0	0	0	0	0	0	0
2	Hunter Rd to Columbia Pk	500	246	146	0	1.5	402	51,100	5	0.051100	0.255500
	Totals	2,045	1,436	197	0		542	68,950		0.068950	0.344750
	FUTURE PLANNING USING THE EXISTING PROP. DENSITY OVER THE UNDEVELOPED ACREAGE	SING THE	EXISTIN	IG PROP.	DENSITY C	VER THI	E UNDE	/ELOP	ED A(SREAGE	
		Total		No. Of	Est. Total	Est. Population	Est. Total			Total Average	Total Peak
Area	Description	Undeveloped	Prop. Density	Estimated	Developed Lots	In Area	_	۹	Peaking	Flow	
Number		Acreage	(Units/ Acre)	Developed Lots	In Area	2.75/unit	In Area	(GPD)	Factor	(MGD)	(MGD)
-	Kittrell Rd and Hunter Rd Intersection	1,189	1.5	1,784		4,907	5,047	642,327	3.25	0.642327	2.087563
	1.A	205	1.5	308	311	846	854	108,675	3.25	0.108675	0.353194
	1.B	541	1.5	812	850	2,232	2,336	297,325	3.25	0.297325	0.966306
	1.C	294	1.5	442	452	1,215	1,242	158,102	3.25	0.158102	0.513832
	1.D	101	1.5	152	152	417	417	53,025	3.25	0.053025	0.172331
	1.E	48	1.5	72	72	198	198	25,200	3.25	0.025200	0.081900
2	Hunter Rd to Columbia Pk	246	1.5	369	515	1,016	1,418	180,413	3.25	0.180413	0.586341
	Totals	1.436		2.154	2,351	5,923	6.464	822,740		0.822740	2.673904

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		Total Peak	Flow	(MGD)	0.155750	0.120750	0.035000	0.159250	0.105000	0.014000	0.040250	0.089250	0.001750	0.008750	0.075250	0.001750	0.001750	0.404250		Total Peak	Flow	(MGD)	0.872804	0.542588	0.330216	0.442374	0.155269	0.067113	0.219993	0.883667	0.146169	0.069331	0.248544	0.112044	0.307580	2.198844	
		Total Average	Flow	(MGD)	0.031130	0.024150	0.007000	0.031850	0.021000	0.002800	0.008050	0.017850	0.0000350	0.001750	0.015050	0.000350	0.000350	0.080850	GE 30	Total Average	Flow	(MGD)	0.268555	0.166950	0.101605	0.136115	0.047775	0.020650	0.067690	0.271898	0.044975	0.021333	0.076475	0.034475	0.094640	0.676568	
			Peaking	ractor		n L	ဂ	2	2	2	2	2	2	2	2	2	2		CREA		Peaking	Factor	3.25	3.25	3.25	3.25	3.25	3.25	3.25	3.25	3.25	3.25	3.25	3.25	3.25		
			Ave. Flow	(GPD)	31,130	7,000	000,7	31,850	21,000	2,800	8,050	17,850	350	1,750	15,050	350	350	80.850	OPED A		Ave. Flow	(GPD)	268,555	166,950	101,605	136,115	47,775	20,650	67,690	271,898	44,975	21,333	76,475	34,475	94,640	676,568	
SI		Population	In Area	2.7 3/UIII	Ι.	081	22	250	165	22	63	140	8	14	118	က	3	635	NDEVEL (Est. Total	Population	In Area	2,110	1,312	798	1,069	375	162	532	2,136	353	168	601	271	744	5,316	
SULATION	FLOWS		Prop. Density	(UIIIS/ ACIE)		C. 1	1.5	.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5		R THE U	Est. Population	In Area	2.75/unit	1,865	1,122	743	819	210	140	469	966'	351	154	483	268	741	4,681	
WER CALC	RY SEWER	No. Of Lots	Planned	(Flamming Dept.)		0	Э	0	0	0	0	0	0	0	0	0	0	0	ENSITY OVER THE UNDEVELOPED ACREAGE	Est. Total	ots	In Area	192	477	290		137	59	193	1777	129	61	219	66	270	1,933	
SOUTHWEST BASIN NO. 3 SEWER CALCULATIONS	DEVELOPMENT OF SANITARY SEWER FLOWS	No. Of	Existing	Developed Lots	0	60	70	91	90	8	23	51	-	2	43	-	1	231	PROP. DEI	No. Of	Estimated	Developed Lots	849	408	270	298	77	51	170	726	128	26	176	98	269	1,702	
ST BASIN	DPMENT (Total	Undeveloped	Acreage	040	7/57	180	199	51	8	114	484	82	37	117	65	180	1.135	EXISTING		Prop. Density	(Units/ Acre)		1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5		
OUTHWE	DEVELO		Total	Acreage		000	298	579	200	69	311	615	85	25	155	92	242	2.092	NG THE	Total	Undeveloped	Acreage	452	272	180	199	51	34	114	484	85	37	117	92	180	1,135	
S			Description		Т	4		Goose Creek Bypass	2.A	2.B	2.C	Tom Anderson Rd	3.A	3.B	3.C	3.D	3.E	Totals	FUTURE PLANNING USI		Description		Hunter Rd, Columbia Pk, and Snowbird Hollow Rd	1.A	1.B	Goose Creek Bypass	2.A	2.B	2.C	Tom Anderson Rd	3.A	3.B	3.C	3.D	3.E	Totals	* Includes acreage outside the UGB.
			Area	Mulliber	-			2				8									Area	Number	*			2*				3*							* Includes acrea







